APPENDIX 2

SOUTH AUBURN TRAFFIC AND TRANSPORT ACCESSIBILITY STUDY

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TRAFFIC, TRANSPORT AND ACCESSIBILITY STUDY Planning Proposal to Rezone Land at Auburn Road and Susan Street, Auburn Hyder Consulting Pty Ltd ABN 76 104 485 289 Level 5, 141 Walker Street Locked Bag 6503 North Sydney NSW 2060 Australia Tel: +61 2 8907 9000 Fax: +61 2 8907 9001 www.hyderconsulting.com



AUBURN CITY COUNCIL SOUTH AUBURN PLANNING PROPOSAL

Traffic, Transport and Accessibility Study

Traffic, Transport and Accessibility Study

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Report No		
Date	20August 2013	

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Executive Summary

Auburn City Council is proposing to amend the land use zoning, height of building and floor space ratio (FSR) controls for land in Auburn bounded by Auburn Road, Helena Street, Susan Street, and Beatrice Street (the site),. The proposed amendments will form part of a planning proposal referred to as the South Auburn Planning Proposal (PP).

An Urban Design Study prepared by MG Planning and Group GSA identified that if the proposed increase in residential and mixed use development permitted by the amended controls were achieved there would be an additional 750 dwellings and 13,960 m2 of mixed use podium gross floor area (GFA) for employment uses. The additional development is forecast to generate in the order of 480 to 530 additional vehicle trips in one peak hour.

Hyder has developed the concept of the "core area" which aims to report traffic impact in those parts of the network that are of critical significance to the site. Within the core area four key intersections have been assessed as follows (see Figure 1-2):

- Auburn Road / Beatrice Street;
- Helena Street / Auburn Road;
- Beatrice Street/ Susan Street and
- Helena Street / Susan Street.

The intersection capacity analysis shows that currently all four intersections within the core area operate with good level of service A and B. The Auburn Road / Beatrice Street traffic signalled intersection will be the most significantly impacted, with a forecast level of service C for post development conditions. The remaining three intersections at Auburn Road / Helena Street, Susan Street/ Beatrice Street and Susan Street/ Helena Street will operate with good level of service A for post development conditions, to the year 2021. The proposed development will not adversely impact road and intersection operations in either the morning or afternoon peak periods.

The current public transport near the vicinity of site provides good amenity to its working and residential population. The site is within one km of Auburn Train Station. Auburn Train Station is serviced by the Western Line and ranks 22nd in terms of number of passengers served. Three bus stops are located within 800m walking radius of the site, which provide access to two bus services; routes 908 and 911 operated by Veolia.

The location of the site in close proximity to public transport (particularly the Auburn Train Station) is appropriate in integrating land use and transport provision in a sustainable manner.

South Auburn Planning Proposal-Traffic, Transport and Accessibility Study

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1 Introduction

1.1 Study Purpose

The following document provides a Traffic, Transport and Accessibility Study (hereafter referred to as "the Study") of a planning proposal to amend the land use zoning, height of building and floor space ratio (FSR) controls on a small parcel of land in Auburn bounded by Auburn Road, Helena Street, Beatrice Street and Susan Street (herein referred to as "the site"). The Study has been commissioned by Auburn City Council ('Council') to investigate the potential traffic implications of South Auburn PP on the road network.

The overall outcome of the Study is a Traffic and Transport assessment which determines the impact on the road network arising from the planning proposal.

1.2 Background

Council is currently undertaking a series of studies designed to test the impact of proposed increases to height of building and floor space ratio (FSR) controls contained within the *Auburn Local Environmental Plan (LEP) 2010*. Hyder previously undertook traffic, transport and modelling assessment for the following two projects for Council:

- Auburn LGA Traffic and Transport Study (Auburn LGA Study) in a draft form, Hyder Consulting Pty Ltd, December 2012. The study identifies existing transport and traffic issues and strategic responses for the Auburn Local Government Area.
- Increased Floor Space Ratio Controls for Certain Land Zoned B4 Mixed Use and R4 High Density Residential within the Auburn LGA (FSR PP Study), Hyder Consulting, August 2013. The FSR PP Study determines traffic impact on the strategic road network arising from the proposed increase in FSR in Auburn, Lidcombe, Berala and Regents Park.
- Increased Floor Space Ratio Controls for Certain Land Zoned B4 Mixed Use & R4 High Density Residential within the Auburn LGA (FSR PP Study), Transport and Accessibility Study, Auburn City Council, August 2013. Council has prepared the transport and accessibility elements of the study.

Similar to FSR PP Study, Council is preparing a planning proposal to amend the zoning, height and FSR controls for the site. The proposed zoning amendments are from R3 Medium Density Residential to part B4 Mixed Use and part R4 High Density under the *Auburn LEP 2010*. Council requires a Traffic, Transport and Accessibility Study to assess the impact of the planning proposal.

This report references relevant traffic and modelling data, and accessibility assessments from the FSR PP study reports undertaken by Hyder and Council, and should be read in conjunction with this report.

Figure 1-1 below shows the South Auburn PP site in the context of broader FSR PP sites in Auburn.



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1.3 Study Aims and Objectives

The main purpose of this study is to assess the impact of the proposed amendments to zoning, height and FSR controls on the road network surrounding the site and provide any mitigation measures if required to accommodate the additional traffic generated by the South Auburn PP. The study has assumed 2021 as the future horizon year for its assessment.

Key objectives include:

- Identify any transport issues that the proposed control amendments are likely to cause.
- Identify accessibility of the site considering bus services, bicycle and pedestrian infrastructure.
- Undertake traffic modelling assessment of the following three distinct scenarios including:
 - Existing Condition (Scenario 1). The existing condition represents 2013 traffic conditions to four key intersections in the immediate vicinity to the site.
 - Impact from South Auburn PP alone (Scenario 2). This scenario assesses impact to four key intersections taking into account background growth and additional traffic from South Auburn PP. The scenario 2 assesses forecast traffic impact for year 2021 condition.
 - Cumulative Impact (Scenario 3). This scenario assesses forecast traffic impact for year 2021 condition taking into account cumulative traffic from both South Auburn PP and FSR PP. In the FSR PP Study, Hyder identified seven intersections in Auburn where road capacity improvements will be required to cater for the additional traffic demand. Intersection upgrading works that are proposed to those seven intersections have been re-examined taking into account additional traffic from South Auburn PP.

1.4 Core Area

Hyder has developed the concept of the "core area" which aims to report traffic impact in those parts of the network that are of critical significance to the South Auburn PP. Within the local vicinity of the site it is important to assess intersection capacities and network connectivity in some detail. This will enable a robust assessment of the impact of traffic movements to and from the site on the immediate road network. Hyder has undertaken a detailed modelling assessment of the "core area of impact". This forms the base-line for this level of assessment. The approximate core area is shown in Figure 1-2.

The core area includes four key intersections as follows:

- Auburn Road / Beatrice Street (I-40);
- Helena Street / Auburn Road (1-53);
- Beatrice Street/ Susan Street (I-54) and
- Helena Street / Susan Street (I-55).

In conjunction with the core area defined above, Hyder has also assessed impact of South Auburn PP within the "wider area" where intersection upgrading works are proposed arising from the FSR PP. The following seven intersections are reassessed within wider area including:

Park Road / Wellington Road (I-9);

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- Park Road / Vaughan Street (I-10);
- Olympic Drive / Vaughan Street (I-13);
- Station Road / Rawson Street (I-22);
- Rawson Street / South Parade (I-39);
- Station Road / Kerr Parade / Civic Road (I-47);and
- Queen Street / Auburn Road / Civic Road (I-49).

Hyder's analysis has found that impact of South Auburn PP to wider area is very small.

Figure 1-2 also shows location of key intersections within the wider area where intersection upgrades are proposed as part of FSR PP study.



Figure 1-2 Definition of Core and Wider Areas and Key Intersections

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1.5 Reference Traffic Data and Model

For the purpose of this study, Hyder undertook a data gap analysis based on available data in relevant studies. Traffic survey was previously conducted in July 2012 for the FSR PP study. Counts at the Auburn Road / Beatrice Street intersection from July 2012 survey were used in Auburn PP study.

The data gap analysis suggested that new traffic counts were required at three key intersections as follows (see previous Figure 1-2):

- Helena Street / Auburn Road (I-53);
- Beatrice Street/ Susan Street (I-54) and
- Helena Street / Susan Street (I-55).

The new traffic survey was undertaken by Skyhigh in June 2013 for both AM and PM peak periods. For modelling intersections Hyder used SIDRA software (version 5.1.13.2093).

1.6 Report Structure

The report has the following chapters:

- Section 1 provides an overview of the Study, background information, study objectives, traffic investigation area, traffic data and software used.
- Section 2 provides an overview of the existing and the potential capacity of residential and mixed use podium GFA proposed at South Auburn.
- Section 3 details transport and accessibility assessment of the site. This section details travel patterns, and examines public transport, walking and cycling facilities relevant to the site.
- Section 4 provides an assessment of forecast traffic impact to road network.
- Section 5 provides conclusions of the Study.

2 The Proposal

The proposal is to rezone the site from its current zoning of R3 medium Density Residential to B4 Mixed Use and R4 High Density Residential. Figure 2-1 shows the proposed zoning map for the site. MG Planning and Group GSA determined the existing and potential future maximum yield for site based on the South Auburn PP. Currently the site contains 93 dwellings and approximately 2,444 m² of mixed use podium gross floor area (GFA) for employment uses. Using the RMS Guide to Traffic Generating Developments, Issue 2.2, October 2002, it is estimated that currently the site generates between 71 and 84 trips in one peak hour.

MG Planning and Group GSA have identified potential yields of the site. Two options were examined as follows:

- Option 1A additional 855 dwellings and 6,980 m² of mixed use podium gross floor area (GFA) for employment uses; and
- Option 1B additional 750 dwellings and 13,960 m² of mixed use podium gross floor area (GFA) for employment uses.

When trip generation was considered, Option 1B will provide the maximum traffic impact to road network. The additional development yield of 750 dwellings and 13,960m² of mixed use podium gross floor area (GFA) in Option 1B have been used for traffic impact assessment purpose.



Figure 2-1 South Auburn Planning Proposal Zoning Map (Sourced from Council)

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3 Transport and Accessibility Assessment

3.1 Journey to Work (JTW)

Data from the Australian Bureau of Transport Statistics (BTS) was used to examine the travel patterns of South Auburn residents. Journey to work data is collected by the Australian Bureau of Transport Statistics as part of the Australian national census. The latest Journey to Work (JTW) for 2011 has been used. The dataset estimates trip purposes. Work trip origin and destinations are coded to the 2011 travel zones. For JTW trips, the study area was determined by the travel zone boundary of 1311 as shown in Figure 3-1.

South Auburn as Workplace

Table 3-1 shows location of residence of people who work in South Auburn. The data shows that majority of the people working in South Auburn live in nearby places such as Auburn, Parramatta, Blacktown and Bankstown. Approximately 26% of residents commuting to South Auburn reside in Auburn, 10% in Merrylands and about 7% in Parramatta.

Place of Residence	Percentage
Auburn	26%
Merrylands – Guildford	10%
Parramatta	7%
Bankstown	6%
Blacktown	5%
Strathfield - Burwood - Ashfield	5%
Fairfield	4%
Baulkham Hills	3%
Mount Druitt	3%
Canada Bay Source: JTW2011	2%

Table 3-1 Residence of People Working in South Auburn



Figure 3-1 Travel Zone Boundary for South Auburn PP

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South Auburn as Residence

Journey to work analysis also revealed that majority of people residing in South Auburn work within Auburn (about 24%). Approximately 18% of South Auburn residents work in City and about 6% in Parramatta (Table 3-2).

Workplace	n of roopio noording in could rive	Percentage
Auburn		24%
Sydney Inner City		18%
No usual address		6%
Parramatta		6%
Bankstown		5%
Strathfield - Burwood - Ashfield		5%
Merrylands - Guildford		5%
Ryde - Hunters Hill		4%
Chatswood - Lane Cove		3%
Canada Bay		2%
Source: JTW2011		

Table 3-2 Workplace Destination of People Residing in South Auburn

3.1.1 Mode Share

In order to establish an approximation of the existing transport mode share of South Auburn residents, the JTW dataset was also analysed for transport mode. Figure 3.-2 show the mode share for work related trips in the South Auburn Area.

The analyses showed that majority of work related trips to/from South Auburn were made by private car (approximately 56 % to 72 %). About 26% of trips from South Auburn were made by train. In comparison only 9% of trips to South Auburn were made by train. Bus accounted for less than 1% trips. The data indicates that private cars and train are the two dominant mode of transport for work related trips to and from South Auburn area.

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3.2 Public Transport

3.2.1 Bus Service

The site is located within 1 km of Auburn Town Centre and Auburn Train Station. The site has good access to existing Auburn Train Station and bus services. The study area is serviced by two bus services; 908 and 911 operated by Veolia. The buses operate in every 30 minutes to one hour in morning and afternoon peak period (Table 3-3) from South Auburn to Auburn Train Station, Auburn Town Centre and Auburn Hospital.

Three bus stops are located within an 800m walking radius of the site (Figure 3-3). Bus stops are located on Auburn Road and Beatrice Street.



Table 3-3 Existing Bus Services in South Auburn

Figure 3-3 Existing Bus Stops in South Auburn

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3.2.2 Rail Service

The site in is within 1 km of Auburn Train Station. Auburn Train Station is serviced by the Western Line and ranks 22nd in terms of number of passengers served (sourced from FSR PP). It provides significant facility for passengers travelling to work in Sydney's CBD locations. The analysis also found that walking was the dominant mode for access to Auburn Train Station as utilisation of other modes such as bus and bicycle was extremely limited.

3.2.3 Cycling and Walking

There are no dedicated on or off road cycling facilities provided within the South Auburn Area. Footpaths are provided on all streets within the subject South Auburn area. Pedestrian crossing is provided at the Auburn Road / Beatrice Road traffic signals on all approaches as shown in Figure 3-4. In the vicinity of the site, no new bicycle route is proposed.



Figure 3-4 Existing Walking Facilities in South Auburn

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4 Traffic Impact Assessment

4.1 Traffic Generation

Table 4.9

The impact assessment is based on estimated yields of 750 dwellings and 13,960 m² of mixed use podium GFA. The trip generation has been estimated using the RMS Guide to Traffic Generating Developments, Issue 2.2, October 2002. Table 4-1 below summarises RMS's trip generation rate and associated assumptions used in this Study.

Table 4-1 RMS's Standard Trip Generations Rates						
Land Use	RTA trip generations rates					
Residential:	High density dwelling: 0.29 peak hour trips per dwelling.					
Employment use: Retail	 For Auburn Town Centre, for retail development PM peak trip generation is 4.6 vehicle trips per hour per 100 m² of GLFA. This rate is proposed for retail GLFA higher than 30,000 m². 					

Employment use: Office Trip generation is 2 vehicle trips per hour per 100 m² of GFA. and commercial

Notes: In AM peak retail trip generation is expected to be significant lower than in PM peak. In AM peak retail trip generation rate is assumed to be about 40% lower than PM peak. About 100 m2 of GFA is equal to 75 m2of Gross Leasable Floor Area (GLFA).

Table 4-2 summarises forecast trip generation from proposed site at South Auburn.

Farment Trin Conservation

Area	Dwellings	Mixed use	Al	M Peak 1 F (Vehicles		PM Peak 1 Hour (Vehicles)		
Alea	Dwennigs	podium GFA (m ²)	In	Out	Two Way	In	Out	Two Way
South Auburn	750	13960	238	243	481	264	270	534

Of the total GFA, about 65% is assumed to be retail and 35% to be office and commercial use (source: Auburn Town Centre Retail and Economic Study 2008).

The additional development proposed for the site is forecast to generate in the order of 480 to 535 vehicle trips in one peak hour. The afternoon peak hour trip generation is about 11% higher than morning peak due to higher proportion of retail trips.

For the purpose of this assessment, it was assumed that traffic generated by zone B4 (west side) will use Auburn Road / Beatrice Street traffic signal and Auburn Road / Helena Street roundabout to enter and exit the B4 zone. All traffic generated by the R4 Zone (east side) will use the Susan Street / Beatrice Street and Susan Street/ Helena Street to enter and exit the R4 zone. Figure 4-1 below shows assumed trip distribution to and from site.

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Figure 4-1 Assumed Trip Distribution to and from Site

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4.2 Impact on Road Network

The future development capacity of the B4 and R4 zones will be determined by the spare capacity available at key intersections providing access to the site. The potential development capacity identified in South Auburn PP will impact four intersections within the core area as follows.

- Auburn Road / Beatrice St;
- Auburn Road / Helena Street;
- Susan Road / Beatrice Street; and
- Helena Street / Susan Street.

The previous Figure 1-2 shows 4 key intersections in the core area.

In general, network impacts from potential development will decline with greater distance form site. The following three scenarios were assessed including:

- Existing Condition (Scenario 1). The existing condition represents 2013 traffic conditions to four key intersections in the immediate vicinity to the site.
- Impact from the South Auburn PP alone (Scenario 2). This scenario assesses impact to four key intersections taking into account background growth and additional traffic from South Auburn PP. The scenario 2 assesses forecast traffic impact for year 2021 condition.
- Cumulative Impact (Scenario 3). The scenario 3 assesses forecast traffic impact for year 2021 condition taking into account cumulative traffic from both South Auburn PP and FSR PP. In FSR PP Study, Hyder identified seven intersections in Auburn where road capacity improvements will be required to cater for the additional traffic demand. Intersection upgrading works that are proposed to those seven intersections have been re-examined taking into account additional traffic from South Auburn PP.

Appendix A included existing turning volumes at four intersections within the core area.

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4.2.1 Assessment Criteria

Intersection Levels of Service (LoS) was assessed using the standard RMS Level of Service criteria for intersections (see Table 4-3 below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
A	<14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
Ε	57 to 70	At capacity; at signals, incidents will cause excessive delays. Roundabouts require other control mode	At capacity, requires other control mode
F	>70	Unsatisfactory with excessive queuing	Unsatisfactory with excessive queuing

Table 4-3 LoS Criteria for Intersection Capacity Analysis

Source: RMS Guide to Traffic Generating Developments

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4.2.2 Existing (2013) Conditions – Scenario 1

 Table 4-4 shows forecast AM and PM peak LoS results for "existing" scenario for year 2013.

 Detailed SIDRA results are included in Appendix A.

ID Ref	Pof	Intersection	Control		Existing AM (2013)			Existing PM (2013)		
	ILGI	mersection	Туре	DoS	LoS	Avg Delay (sec)	DoS	LoS	Avg Delay (sec)	
1	I-40	Auburn Road / Beatrice St	Signal	0.6	В	26	0.7	В	27	
2	I-53	Auburn Road / Helena St	Roundabout	0.3	А	8	0.3	A	7	
3	I-54	Susan St/ Beatrice St	Roundabout	0.3	A	8	0.2	A	7	
4	I-55	Susan St / Helena St	Priority	0.1	A	4	0.2	A	2	

Table 4-4 Performance Results for Existing (2013) Conditions

Source: F:\AA005093\SouthAuburnPP\Modelling\SIDRA\Existing_2013.sip

The intersection capacity analysis results in Table 4-4 shows that currently all four intersections within the core area operate with good level of service A and B. The results indicate that (under the current layout and control) all four intersections will have spare capacity for future growth.

4.2.3 Impact from South Auburn PP- Scenario 2

The additional traffic generated from the South Auburn PP has been assigned to the road network. Consistent with FSR PP study a background traffic growth of 0.5 % per annum is used. All four intersections were assessed for year 2021 taking into account both background traffic growth and additional traffic from South Auburn PP. The analysis was undertaken for both AM and PM peak traffic conditions.

Table 4-5 shows the intersection performance results for year 2021 for post development traffic condition. Detailed SIDRA results are included in Appendix A.

			Oraclast	San Par	A	M	PM		
ID	Ref	Intersection	Control Type	DoS	LoS	Avg Delay (sec)	DoS	LoS	Avg Delay (sec)
1	I-40	Auburn Road / Beatrice St	Signal	0.7	С	34	0.84	С	36
2	I-53	Auburn Road / Helena St	Roundabout	0.2	A	8	0.25	A	8
3	I-54	Susan St/ Beatrice St	Roundabout	0.1	A	8	0.19	А	8
4	I-55	Susan St / Helena St	Priority	0.1	А	9	0.2	A	10

Table 4-5 Forecast Level of Service with South Auburn PP – Scenario 2

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The level of service analysis in Table 4-5 indicates that all four intersections assessed will accommodate additional traffic generated by the proposed development. The Auburn Road with Beatrice Street traffic signals will have maximum impact and level of service is forecast C for post development condition. The remaining three intersections at Auburn Road / Helena Street, Susan Street/ Beatrice Street and Susan Street/ Helena Street will operate with good level of service A for post development condition for year 2021.

The modelling results of four intersections for post development condition do not suggest the need for any potential upgrading works. The additional development traffic to and from site will have small impact to road and intersection operation within the core area.

4.2.4 Cumulative Impact – Scenario 3

The previous Section 4.2.3 has showed small impact to four intersections within the core area. The 7 intersections within the wider area have been re-examined for improved intersection condition taking into account cumulative traffic from both South Auburn PP and FSR PP. The modelling analysis has found that impact from proposed site will decline with greater distance from the core area. The proposed intersections improvements identified in FSR PP study would accommodate additional traffic generated by the South Auburn PP.

5 Conclusions

This Traffic, Transport and Accessibility Study has been prepared by Hyder Consulting Pty Ltd (Hyder) to support the conditions of the gateway determination of a Planning Proposal to rezone a parcel of land in Auburn bounded by Auburn Road, Helena Street, Beatrice Street and Susan Street. The main purpose of this Study is to assess the impact of the proposed rezoning on the road network and provide any required mitigation measures to accommodate the additional traffic generated by the South Auburn PP. The Study has assumed 2021 as the future horizon year for its assessment. The MG Planning and Group GSA Urban Design Study identified that if the proposed increase in residential and mixed use densities were achieved there would be an additional 750 dwellings and 13,960 m2 of mixed use podium gross floor area (GFA) for employment uses.

The current public transport near the vicinity of proposed site provides good level of access for the working and residential population of the site. The site is within 1 km of Auburn Train Station. Auburn Train Station is serviced by the Western Line and ranks 22nd in terms of number of passengers served.

The catchment area of proposed site is serviced by two bus services; 908 and 911 operated by Veolia. The buses operate every 30 to 60 minutes in morning and afternoon peak period from South Auburn to the Auburn Train Station, the Auburn Town Centre and Auburn Hospital. . Three bus stops are located within an 800m walking radius of the site.

The location of site in close proximity to public transport (particularly the Auburn Train Station) is appropriate in integrating land use and transport provision in a sustainable manner.

Hyder has developed the concept of the "core area" which aims to report traffic impact in those parts of the network that are of critical significance to the South Auburn PP. Within the core area four key intersections have been assessed as follows (see Figure 1-2):

- Auburn Road / Beatrice Street (1-40);
- Helena Street / Auburn Road (I-53);
- Beatrice Street/ Susan Street (I-54) and
- Helena Street / Susan Street (I-55).

The intersection capacity analysis shows that currently all four intersections within the core area operate with good level of service A and B. The potential development of the Planning Proposal will have the forecast impact of reducing the level of service of the Auburn Road with Beatrice Street intersection to C. The remaining three intersections at Auburn Road / Helena Street, Susan Street/ Beatrice Street and Susan Street/ Helena Street will operate with good level of service A.

The modelling results of four intersections for potential development conditions do not suggest the need for any potential upgrading works. The additional traffic to and from site will only have small impact on the road and intersection operation within the core area. Beyond the core area, traffic analysis has identified only a minor impact from the South Auburn PP.

APPENDIX A

TRAFFIC GENRATED BY SOUTH AUBURN PP AND DETAILED SIDRA RESUTLS

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DETAILED SIDRA RESULTS EXISTING 2013

AUBURN ROAD / BEATRICE STREET MOVEMENT SUMMARY Site: 2013_Existing_I-40 AuburnRoad/BeatriceSt_AM

1-40

Auburn Rd/Beatrice St Signals - Fixed Time Cycle Time = 55 seconds (Optimum Cycle Time - Minimum Delay)

Mov ID	Turn	Demand	HV C	leg. Satn	Average	Level of	95% Back of Queu		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m	A approximation	per veh	km/h
South: A	uburn F	Rd(S)						1			
1	L	108	0.0	0.321	28.6	LOS C	2.7	18.6	0.91	0.77	29.6
2	Т	124	0.8	0.623	25.2	LOS B	4.8	34.4	0.98	0.83	29.3
3	R	53	6.0	0.623	31.8	LOS C	4.8	34.4	0.98	0.85	29.2
Approact	h	285	1.5	0.623	27.7	LOS B	4.8	34.4	0.95	0.81	29.4
East: Be	atrice S	t (E)						1.5			
4	L	33	3.2	0.127	26.7	LOS B	1.1	7.7	0.85	0.74	30.8
5	Т	168	0.0	0.590	23.5	LOS B	5.0	35.2	0.95	0.79	30.2
6	R	37	0.0	0.590	30.2	LOS C	5.0	35.2	0.96	0.83	30.0
Approacl	h	238	0.4	0.590	25.0	LOS B	5.0	35.2	0.94	0.79	30.2
North: Au	uburn R	d (N)									
7	L	54	0.0	0.159	27.7	LOS B	1.3	8.9	0.88	0.73	30.0
8	Т	88	2.4	0.512	24.3	LOS B	3.5	24.8	0.96	0.76	29.7
9	R	44	0.0	0.512	30.7	LOS C	3.5	24.8	0.96	0.80	29.5
Approach	h	186	1.1	0.512	26.8	LOS B	3.5	24.8	0.93	0.76	29.7
West: Be		St (W)									
10	L	42	0.0	0.591	28.1	LOS B	2.7	18.9	0.89	0.82	30.6
11	Т	233	1.4	0.591	22.7	LOS B	5.1	36.2	0.94	0.78	30.6
12	R	29	7.1	0.591	29.7	LOS C	5.1	36.2	0.96	0.84	30.3
Approach	n	304	1.7	0.591	24.1	LOS B	5.1	36.2	0.93	0.79	30.6
All Vehic		1014	1.2	0.623	25.8	LOS B	5.1	36.2	0.94	0.79	30.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

N.K.	Description	Demand	Average Delay	Level of Service	Average Back	of Queue	Prop. Queued	Effective Stop Rate
Mov ID		Flow			Pedestrian	Distance		
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	21.8	LOS C	0.1	0.1	0.89	0.89
P3	Across E approach	53	21.8	LOS C	0.1	0.1	0.89	0.89
P5	Across N approach	53	21.8	LOS C	0.1	0.1	0.89	0.89
P7	Across W approach	53	21.8	LOS C	0.1	0.1	0.89	0.89
Il Pedestrians		212	21.8	LOS C			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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MOVEMENT SUMMARY

Site: 2013_Existing__I-40 AuburnRoad/BeatriceSt_PM

I-40 Auburn Rd/Beatrice St

Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Mov ID	Tum	Demand Flow	HV Deg. Satn		Average Deiay	Level of Service	95% Back of Queue Vehicles Distance			Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		venioleo	m		per veh	km/h
South: A	uburn F	Rd(S)					Sec. 201 (SQ.2)	E. S.			
1	L	34	0.0	0.143	29.8	LOS C	0.9	6.2	0.85	0.71	29.1
2	Т	119	0.0	0.365	25.2	LOS B	3.7	26.2	0.91	0.72	29.7
3	R	12	9.1	0.365	31.8	LOS C	3.7	26.2	0.91	0.81	29.5
Approac	h	164	0.6	0.365	26.6	LOS B	3.7	26.2	0.89	0.73	29.6
East: Be	atrice S	t (E)									
4	L	34	0.0	0.156	25.6	LOS B	1.5	10.4	0.78	0.75	31.4
5	Т	292	1.1	0.721	25.5	LOS B	10.0	70.7	0.95	0.86	29.4
6	R	55	0.0	0.721	32.5	LOS C	10.0	70.7	0.97	0.92	29.0
Approach 380		380	0.8	0.721	26.5	LOS B	10.0	70.7	0.94	0.86	29.5
North: A		d (N)									
7	L	58	0.0	0.169	30.3	LOS C	1.7	12.0	0.86	0.74	29.0
8	Т	111	2.9	0.681	29.1	LOS C	6.3	45.0	0.98	0.86	27.5
8 9	R	91	0.0	0.681	35.8	LOS C	6.3	45.0	0.98	0.88	27.4
Approac	h	259	1.2	0.681	31.7	LOS C	6.3	45.0	0.95	0.84	27.8
Nest: Be	eatrice S	St (VV)									
10	L	109	1.0	0.614	27.6	LOS B	2.8	20.1	0.81	0.80	30.0
11	Т	151	1.4	0.404	22.8	LOS B	4.7	33.6	0.88	0.72	30.7
12	R	22	0.0	0.404	29.3	LOS C	4.7	33.6	0.88	0.82	30.4
Approac	h	282	1.1	0.614	25.2	LOS B	4.7	33.6	0.85	0.76	30.4
All Vehic		1085	1.0	0.721	27.4	LOS B	10.0	70.7	0.91	0.81	29.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

	Description	Demand	Average Delay	Level of Service	Average Back	of Queue	Prop. Queued	Effective Stop Rate
Mov ID		Flow			Pedestrian	Distance		
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	26.8	LOS C	0.1	0.1	0.91	0.9
P3	Across E approach	53	26.8	LOS C	0.1	0.1	0.91	0.9
P5	Across N approach	53	26.8	LOS C	0.1	0.1	0.91	0.9
P7	Across W approach	53	26.8	LOS C	0.1	0.1	0.91	0.9
Il Pedestrians		212	26.8	LOS C			0.91	0.9

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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AUBURN ROAD / HELENA STREET MOVEMENT SUMMARY Site: 2013_Existing_I-53 Auburn Road/Helena

Auburn Road_Helena Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: A	Auburn F	Road (S)	10 A 10 A						54		
1	L	35	3.0	0.229	7.2	LOS A	1.4	10.2	0.48	0.61	41.6
2	Т	136	0.8	0.229	6.3	LOS A	1.4	10.2	0.48	0.55	41.8
2 3	R	68	3.1	0.229	10.5	LOS A	1.4	10.2	0.48	0.74	39.8
Approac	h	239	1.8	0.229	7.6	LOS A	1.4	10.2	0.48	0.61	41.2
East: He	elena St	reet (E)									
4	L	28	7.4	0.199	7.4	LOS A	1.2	8.7	0.48	0.61	42.2
5	Т	108	1.9	0.199	6.3	LOS A	1.2	8.7	0.48	0.55	42.4
6	R	67	1.6	0.199	10.5	LOS A	1.2	8.7	0.48	0.74	40.5
Approach 20		204	2.6	0.199	7.8	LOS A	1.2	8.7	0.48	0.62	41.7
		toad (N)									
7	L	48	2.2	0.214	7.7	LOS A	1.3	9.3	0.54	0.65	38.4
8	т	121	0.0	0.214	6.7	LOS A	1.3	9.3	0.54	0.59	38.5
9	R	38	2.8	0.214	11.0	LOS A	1.3	9.3	0.54	0.77	36.2
Approac	h	207	1.0	0.214	7.7	LOS A	1.3	9.3	0.54	0.64	38.0
at an even over a plan over		reet (W)									
10	L	97	0.0	0.318	7.7	LOS A	2.1	14.9	0.56	0.66	30.5
11	т	153	2.8	0.318	6.9	LOS A	2.1	14.9	0.56	0.61	30.7
12	R	64	0.0	0.318	11.0	LOS A	2.1	14.9	0.56	0.77	28.5
Approac	h	314	1.3	0.318	8.0	LOS A	2.1	14.9	0.56	0.66	30.1
All Vehic		964	1.6	0.318	7.8	LOS A	2.1	14.9	0.52	0.64	38.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Street AM

MOVEMENT SUMMARY

Site: 2013_Existing_I-53 Auburn Road/Helena Street_PM

Auburn Road_Helena Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV Deg. Satn		Average	Level of Service	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay		Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: A	Auburn F	Road (S)			1.000		an ann an			1991 - 1997 - 19	
1	L	29	0.0	0.142	7.6	LOS A	0.8	5.8	0.52	0.64	41.5
23	Т	82	1.3	0.142	6.7	LOS A	0.8	5.8	0.52	0.58	41.6
3	R	24	0.0	0.142	10.9	LOS A	0.8	5.8	0.52	0.76	39.5
Approad	ch	136	0.8	0.142	7.7	LOS A	0.8	5.8	0.52	0.63	41.2
East: He	elena St	reet (E)									
4	L	42	0.0	0.271	7.2	LOS A	1.7	12.4	0.50	0.63	42.3
5	Т	202	1.6	0.271	6.4	LOS A	1.7	12.4	0.50	0.57	42.4
6	R	38	0.0	0.271	10.5	LOS A	1.7	12.4	0.50	0.76	40.5
Approach 282		1.1	0.271	7.1	LOS A	1.7	12.4	0.50	0.60	42.1	
North: A	uburn R	oad (N)									
7	L	23	0.0	0.191	7.1	LOS A	1.1	8.1	0.46	0.60	38.7
8	Т	113	0.9	0.191	6.2	LOS A	1.1	8.1	0.46	0.54	38.9
9	R	63	1.7	0.191	10.4	LOS A	1.1	8.1	0.46	0.74	36.6
Approac	h	199	1.1	0.191	7.7	LOS A	1.1	8.1	0.46	0.61	38.1
West: H	elena St	reet (W)									
10	L	23	0.0	0.189	6.6	LOS A	1.2	8.2	0.38	0.57	31.8
11	Т	146	0.7	0.189	5.7	LOS A	1.2	8.2	0.38	0.50	32.4
12	R	44	2.4	0.189	9.9	LOS A	1.2	8.2	0.38	0.74	29.6
Approac	h	214	1.0	0.189	6.7	LOS A	1.2	8.2	0.38	0.56	31.7
All Vehic		831	1.0	0.271	7.2	LOS A	1.7	12.4	0.46	0.60	39.6

Level of Service (LOS) Method: Delay (RTA NSW). Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements. Roundabout Capacity Model: SIDRA Standard. SIDRA Standard Delay Model used.

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South Auburn Planning Proposal-Traffic, Transport and Accessibility Study

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SUSAN STREET / BEATRICE STREET MOVEMENT SUMMARY Susan Street/Beatr

Site: 2013_Existing_I-54 Susan Street/Beatrice Street-AM _AdjCounts

Beatrice Street_Susan Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop. Effec	Effective	ve Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: S	Susan Si	treet (S)									
1	L	24	0.0	0.257	7.4	LOS A	1.6	11.4	0.50	0.64	38.9
2	Т	235	2.2	0.257	6.6	LOS A	1.6	11.4	0.50	0.58	39.1
3	R	3	33.3	0.257	11.5	LOS A	1.6	11.4	0.50	0.82	36.7
Approac	h	262	2.4	0.257	6.7	LOS A	1.6	11.4	0.50	0.59	39.0
East: Be	atrice S	treet (E)									
4	L	14	0.0	0.145	6.7	LOS A	0.8	5.8	0.38	0.58	32.2
5	Т	120	0.0	0.145	5.8	LOS A	0.8	5.8	0.38	0.51	32.8
6	R	25	4.2	0.145	10.1	LOS A	0.8	5.8	0.38	0.75	29.9
Approach		159	0.7	0.145	6.6	LOS A	0.8	5.8	0.38	0.55	32.2
North: S	usan St	reet (N)									
7	L	1	0.0	0.102	6.2	LOS A	0.6	4.2	0.29	0.51	35.0
8	Т	25	0.0	0.102	5.3	LOS A	0.6	4.2	0.29	0.44	35.8
9	R	94	0.0	0.102	9.5	LOS A	0.6	4.2	0.29	0.66	32.2
Approac	h	120	0.0	0.102	8.6	LOS A	0.6	4.2	0.29	0.61	32.9
West: Be	eatrice S	Street (W)									
10	L	258	2.4	0.348	7.8	LOS A	2.4	17.3	0.58	0.66	30.2
11	Т	42	2.5	0.348	6.9	LOS A	2.4	17.3	0.58	0.61	30.3
12	R	46	4.5	0.348	11.1	LOS A	2.4	17.3	0.58	0.75	28.3
Approach	h	346	2.7	0.348	8.1	LOS A	2.4	17.3	0.58	0.66	29.9
All Vehic		887	1.9	0.348	7.5	LOS A	2.4	17.3	0.48	0.61	34.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 2013_Existing_I-54 Susan Street/Beatrice Street-PM_AdjCounts

Beatrice Street_Susan Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV E	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: S	Susan Si	treet (S)									
1	L	16	0.0	0.169	8.1	LOS A	1.0	6.9	0.57	0.68	38.6
2	Т	136	0.0	0.169	7.2	LOS A	1.0	6.9	0.57	0.63	38.7
3	R	2	0.0	0.169	11.4	LOS A	1.0	6.9	0.57	0.81	36.1
Approac	h	154	0.0	0.169	7.4	LOS A	1.0	6.9	0.57	0.64	38.6
East: Be	eatrice S	treet (E)									
4	L	16	0.0	0.222	7.2	LOS A	1.3	9.5	0.48	0.63	31.8
5	Т	207	0.5	0.222	6.4	LOS A	1.3	9.5	0.48	0.57	32.1
6	R	6	0.0	0.222	10.5	LOS A	1.3	9.5	0.48	0.78	29.5
Approac	h	229	0.5	0.222	6.5	LOS A	1.3	9.5	0.48	0.58	32.0
North: S	usan St	reet (N)									
7	L	1	0.0	0.162	6.1	LOS A	1.0	6.9	0.29	0.51	35.1
8	Т	42	0.0	0.162	5.3	LOS A	1.0	6.9	0.29	0.44	35.8
9	R	157	0.0	0.162	9.5	LOS A	1.0	6.9	0.29	0.66	32.3
Approac	h	200	0.0	0.162	8.6	LOS A	1.0	6.9	0.29	0.61	32.9
Nest: Be	eatrice S	treet (W)									
10	L	134	1.6	0.194	6.6	LOS A	1.2	8.7	0.40	0.56	31.5
11	т	57	1.9	0.194	5.7	LOS A	1.2	8.7	0.40	0.49	32.1
12	R	28	0.0	0.194	9.9	LOS A	1.2	8.7	0.40	0.71	29.3
Approac	h	219	1.4	0.194	6.8	LOS A	1.2	8.7	0.40	0.56	31.4
All Vehic		802	0.5	0.222	7.3	LOS A	1.3	9.5	0.43	0.59	33.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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SUSAN STREET / HELENA STREET MOVEMENT SUMMARY Susau

Site: 2013_Existing_I-55 Susan Street/Helena Street_AM

Helena Street_Susan Street: Roundabout: 2013

Giveway / Yield (Two-Way)

Mov ID	Turn	Demand Flow	HV D	leg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: He	elena St	reet (E)	- 1	1. 1923			Sec. Sec.	19 A.	1 N	5	
5	Т	153	0.7	0.136	1.0	LOSA	0.8	5.7	0.39	0.00	43.9
6	R	80	1.3	0.136	7.4	LOSA	0.8	5.7	0.39	0.72	41.2
Approac	h	233	0.9	0.136	3.2	NA	0.8	5.7	0.39	0.25	42.9
North: S	usan St	reet									
7	L	36	2.9	0.048	7.4	LOS A	0.1	0.9	0.29	0.60	28.7
9	R	25	0.0	0.054	11.7	LOS A	0.2	1.4	0.53	0.76	34.6
Approac	h	61	1.7	0.054	9.1	LOS A	0.2	1.4	0.39	0.66	32.2
West: He	elena St	reet (W)									
10	L	153	0.7	0.139	6.4	LOS A	0.0	0.0	0.00	0.71	30.8
11	Т	107	3.9	0.139	0.0	LOSA	0.0	0.0	0.00	0.00	50.0
Approac	h	260	2.0	0.139	3.8	NA	0.0	0.0	0.00	0.42	37.5
All Vehic	les	554	1.5	0.139	4.1	NA	0.8	5.7	0.21	0.37	40.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Page 8

Site: 2013_Existing_I-55 Susan Street/Helena Street_PM

Helena Street_Susan Street: Roundabout: 2013

Giveway / Yield (Two-Way)

Mov ID	Turn	Demand	HV E	leg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	Sec		veh	m		per veh	km/h
East: He	elena St	reet (E)	A Marine	march	a weindigt	diama na tang	- Andrew	See Second		and the state of the	12010
5	Т	233	0.9	0.163	0.8	LOS A	1.0	7.1	0.36	0.00	44.5
6	R	63	0.0	0.163	7.2	LOS A	1.0	7.1	0.36	0.74	41.4
Approac	h	296	0.7	0.163	2.2	NA	1.0	7.1	0.36	0.16	43.8
North: S	Susan St	reet								No. Constant	
7	L	42	0.0	0.055	7.1	LOS A	0.1	1.0	0.26	0.59	28.9
9	R	53	6.0	0.122	12.9	LOS A	0.5	3.4	0.57	0.83	33.6
Approac	h	95	3.3	0.122	10.4	LOS A	0.5	3.4	0.43	0.72	32.3
West: H	elena St	reet (W)									
10	L	91	1.2	0.106	6.4	LOS A	0.0	0.0	0.00	0.75	30.8
11	Т	111	0.0	0.106	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approac	h	201	0.5	0.106	2.9	NA	0.0	0.0	0.00	0.34	40.1
All Vehic	cles	592	1.1	0.163	3.7	NA	1.0	7.1	0.25	0.31	41.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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SCENARIO 2 RESULTS AUBURN ROAD / BEATRICE STREET MOVEMENT SUMMARY

Site: 2021_S2__I-40 AuburnRoad/BeatriceSt_AM

1-40

Auburn Rd/Beatrice St

Signals - Fixed Time Cycle Time = 90 seconds (User-Given Cycle Time)

Mov ID	Turn	Demand	HV C	eg. Satn	Average	Level of	95% Back		Prop. Queued	Effective Stop Rate	Average Speed
		Flow	-		Delay	Service	Vehicles	Distance	Queueu		
N. 19-16	and the	veh/h	%	v/c	Sec	and the second	veh	m	and the state	per veh	km/h
South: A	uburn F	Rd(S)							al a minister		
1	L	191	0.0	0.636	36.4	LOS C	7.0	49.3	0.86	0.80	26.6
2	Т	123	0.8	0.708	38.2	LOS C	10.1	72.0	0.98	0.88	24.6
3	R	110	2.7	0.708	44.7	LOS D	10.1	72.0	0.98	0.89	24.5
Approac	h	424	0.9	0.708	39.1	LOS C	10.1	72.0	0.93	0.85	25.5
East: Be	atrice S	t (E)									
4	L	32	3.1	0.097	26.8	LOS B	0.9	6.6	0.68	0.70	30.4
5	Т	201	0.0	0.429	26.1	LOS B	8.3	58.2	0.83	0.70	29.4
6	R	37	0.0	0.429	32.5	LOS C	8.3	58.2	0.83	0.84	28.9
Approac	h	270	0.4	0.429	27.1	LOS B	8.3	58.2	0.82	0.72	29.4
North: A		d (N)									
7	L	53	0.0	0.134	33.3	LOS C	1.8	12.4	0.79	0.73	27.7
8	Т	150	1.3	0.539	35.0	LOS C	7.8	55.0	0.94	0.78	25.9
9	R	44	0.0	0.539	41.4	LOS C	7.8	55.0	0.94	0.83	25.7
Approac	h	247	0.8	0.539	35.8	LOS C	7.8	55.0	0.91	0.77	26.2
West: Be											
10	L	42	0.0	0.689	32.3	LOS C	3.5	24.7	0.72	0.87	28.6
11	Т	239	1.3	0.689	29.9	LOSC	12.6	89.6	0.88	0.78	27.5
12	R	137	1.5	0.689	37.8	LOS C	12.6	89.6	0.94	0.87	26.7
Approac		418	1.2	0.689	32.7	LOS C	12.6	89.6	0.88	0.82	27.3
All Vehic		1359	0.9	0.708	34.1	LOSC	12.6	89.6	0.89	0.80	26.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	. Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec	Sector Sector	ped	m		per ped
P1	Across S approach	53	39.2	LOS D	0.1	0.1	0.93	0.9
P3	Across E approach	53	39.2	LOS D	0.1	0.1	0.93	0.9
P5	Across N approach	53	39.2	LOS D	0.1	0.1	0.93	0.9
P7	Across W approach	53	39.2	LOS D	0.1	0.1	0.93	0.9
I Pede	estrians	212	39.2	LOS D			0.93	0.9

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



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South Auburn Planning Proposal—Traffic, Transport and Accessibility Study Hyder Consulting Pty Ltd-ABN 76 104 485 289

Site: 2021_S2__I-40 AuburnRoad/BeatriceSt_PM

1-40

Auburn Rd/Beatrice St Signals - Fixed Time Cycle Time = 80 seconds (Practical Cycle Time)

Mov ID	Tum	Demand	HV D	leg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: /	Auburn F	Rd(S)									
1	L	157	0.0	0.754	38.2	LOS C	5.7	39.8	0.86	0.90	26.0
2	Т	118	0.0	0.658	33.9	LOS C	7.7	53.8	0.97	0.84	26.0
3	R	82	1.2	0.658	40.4	LOS C	7.7	53.8	0.97	0.86	25.9
Approad	ch	357	0.3	0.754	37.3	LOS C	7.7	53.8	0.92	0.87	26.0
East: Be	eatrice S	t (E)									
4	L	33	0.0	0.146	26.6	LOS B	1.4	10.0	0.73	0.74	30.8
5	Т	298	1.0	0.678	28.4	LOS B	12.0	84.6	0.93	0.81	28.3
6	R	54	0.0	0.678	35.4	LOS C	12.0	84.6	0.95	0.87	27.9
Approad	ch	385	0.8	0.678	29.2	LOS C	12.0	84.6	0.92	0.81	28.4
North: A	uburn R	d (N)									
7	L	57	0.0	0.197	32.1	LOS C	2.6	18.2	0.82	0.76	28.5
8	Т	173	1.7	0.793	37.0	LOSC	10.0	70.4	0.97	0.92	25.0
9	R	90	0.0	0.793	45.5	LOS D	10.0	70.4	1.00	0.97	24.4
Approad	ch	320	0.9	0.793	38.5	LOS C	10.0	70.4	0.95	0.91	25.4
West: B	eatrice S	St (W)									
10	L	109	0.9	0.683	31.2	LOS C	3.4	24.0	0.75	0.83	28.6
11	Т	183	1.1	0.843	40.6	LOS C	13.2	93.1	1.00	1.05	23.9
12	R	116	0.0	0.843	47.1	LOS D	13.2	93.1	1.00	1.05	23.9
Approad	h	408	0.7	0.843	39.9	LOS C	13.2	93.1	0.93	0.99	25.0
All Vehic		1470	0.7	0.843	36.2	LOSC	13.2	93.1	0.93	0.90	26.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m	AL ACTION OF A	per ped
P1	Across S approach	53	34.2	LOS D	0.1	0.1	0.93	0.93
P3	Across E approach	53	34.2	LOS D	0.1	0.1	0.93	0.93
P5	Across N approach	53	34.2	LOS D	0.1	0.1	0.93	0.93
P7	Across W approach	53	34.2	LOS D	0.1	0.1	0.93	0.93
Il Pede	estrians	212	34.2	LOS D			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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AUBURN ROAD / HELENA STREET MOVEMENT SUMMARY Site: 2021_S2_I-53 Auburn Road/Helena Street_AM

Auburn Road_Helena Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV C	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	Sec		veh	m		per veh	km/h
South: A	Auburn F	Road (S)									
1	L	34	2.9	0.269	7.3	LOS A	1.7	12.3	0.50	0.62	41.6
23	Т	176	0.6	0.269	6.4	LOS A	1.7	12.3	0.50	0.57	41.7
3	R	69	2.9	0.269	10.6	LOS A	1.7	12.3	0.50	0.75	39.8
Approac	h	279	1.4	0.269	7.5	LOS A	1.7	12.3	0.50	0.62	41.2
East: He	elena St	reet (E)								0	
4	L	31	6.5	0.208	7.5	LOS A	1.3	9.1	0.51	0.63	42.1
5	Т	110	1.8	0.208	6.5	LOS A	1.3	9.1	0.51	0.57	42.2
6	R	67	1.5	0.208	10.7	LOS A	1.3	9.1	0.51	0.75	40.3
Approac	h	208	2.4	0.208	8.0	LOS A	1.3	9.1	0.51	0.64	41.6
North: A		oad (N)									
7	L	48	2.1	0.244	7.7	LOS A	1.5	10.9	0.55	0.66	38.3
8	Т	144	0.0	0.244	6.8	LOS A	1.5	10.9	0.55	0.60	38.4
9	R	44	2.3	0.244	11.0	LOS A	1.5	10.9	0.55	0.77	36.2
Approac	h	236	0.8	0.244	7.8	LOS A	1.5	10.9	0.55	0.65	38.0
		reet (W)									
10	L	104	1.0	0.338	8.1	LOS A	2.3	16.1	0.61	0.69	30.2
11	т	152	2.6	0.338	7.3	LOS A	2.3	16.1	0.61	0.64	30.3
12	R	64	0.0	0.338	11.4	LOS A	2.3	16.1	0.61	0.79	28.1
Approac		320	1.6	0.338	8.4	LOS A	2.3	16.1	0.61	0.69	29.8
All Vehic		1043	1.5	0.338	7.9	LOS A	2.3	16.1	0.55	0.65	38.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 2021_S2_I-53 Auburn Road/Helena Street_PM

Auburn Road_Helena Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV C	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: A	Auburn A	Road (S)				in the second		1910	and the second		a constant
1	L	29	0.0	0.176	7.7	LOS A	1.1	7.4	0.54	0.65	41.4
23	Т	110	0.9	0.176	6.9	LOS A	1.1	7.4	0.54	0.60	41.5
3	R	27	0.0	0.176	11.0	LOS A	1.1	7.4	0.54	0.78	39.5
Approad	ch	166	0.6	0.176	7.7	LOS A	1.1	7.4	0.54	0.64	41.1
East: He	elena St	reet (E)									
4	L	43	0.0	0.288	7.7	LOS A	1.9	13.3	0.56	0.66	42.0
5	Т	201	1.5	0.288	6.9	LOS A	1.9	13.3	0.56	0.61	42.1
6	R	38	0.0	0.288	11.0	LOS A	1.9	13.3	0.56	0.78	40.2
Approac	ch	282	1.1	0.288	7.5	LOS A	1.9	13.3	0.56	0.64	41.8
North: A	uburn R	load (N)									
7	L	23	0.0	0.246	7.2	LOS A	1.6	11.0	0.49	0.62	38.6
8	Т	160	0.6	0.246	6.3	LOS A	1.6	11.0	0.49	0.56	38.8
9	R	73	1.4	0.246	10.5	LOS A	1.6	11.0	0.49	0.75	36.5
Approac	h	256	0.8	0.246	7.6	LOS A	1.6	11.0	0.49	0.62	38.1
Nest: H	elena Si	treet (W)									
10	L	33	0.0	0.206	6.8	LOS A	1.3	9.0	0.43	0.59	31.5
11	Т	147	0.0	0.206	5.9	LOS A	1.3	9.0	0.43	0.52	32.0
12	R	44	2.3	0.206	10.2	LOS A	1.3	9.0	0.43	0.74	29.5
Approac	h	224	0.4	0.206	6.9	LOS A	1.3	9.0	0.43	0.58	31.3
All Vehic		928	0.8	0.288	7.4	LOS A	1.9	13.3	0.51	0.62	39.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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SUSAN STREET / BEATRICE STREET MOVEMENT SUMMARY Site: 2021_S2_I-Street/Beatrice

Site: 2021_S2_I-54 Susan Street/Beatrice Street-AM _AdjCounts

Beatrice Street_Susan Street: Roundabout: 2013

Roundabout

Mov ID	Tum	Demand	HV D	eg. Satn	Average	Level of Service	95% Back		Prop. Queued	Effective Stop Bate	Average
		Flow			Delay	Service	Vehicles	Distance	Queueu	Stop Rate	Speed
0		veh/h	%	v/c	Sec		veh	m	N.S. S. S. C. M.	per veh	km/h
South: S	Susan S	treet (S)	5/24/24/22	N71343	Same A Make	11. 18 H. M.	What the solution			Contraction of the second	and the second
1	L	58	0.0	0.311	7.4	LOS A	2.0	14.5	0.52	0.65	38.8
2	Т	260	1.9	0.311	6.6	LOS A	2.0	14.5	0.52	0.59	38.9
3	R	3	33.3	0.311	11.6	LOS A	2.0	14.5	0.52	0.82	36.6
Approac	ch	321	1.9	0.311	6.8	LOS A	2.0	14.5	0.52	0.60	38.9
East: Be	eatrice S	street (E)									
4	L	14	0.0	0.146	6.8	LOS A	0.8	5.8	0.40	0.59	32.1
5	Т	119	0.0	0.146	5.9	LOS A	0.8	5.8	0.40	0.52	32.6
6	R	25	4.0	0.146	10.2	LOS A	0.8	5.8	0.40	0.75	29.8
Approac	h	158	0.6	0.146	6.7	LOS A	0.8	5.8	0.40	0.56	32.1
******	usan St	reet (N)									
7	L	1	0.0	0.108	6.2	LOS A	0.6	4.5	0.31	0.52	35.0
8	Т	32	0.0	0.108	5.4	LOS A	0.6	4.5	0.31	0.44	35.6
9	R	93	0.0	0.108	9.5	LOS A	0.6	4.5	0.31	0.66	32.2
Approac		126	0.0	0.108	8.4	LOS A	0.6	4.5	0.31	0.61	33.0
		Street (W)		51.0		1	100	100			
10	1	314	1.9	0.421	8.1	LOS A	3.1	22.1	0.64	0.69	29.8
11	Т	42	2.4	0.421	7.3	LOS A	3.1	22.1	0.64	0.65	29.8
12	R	54	3.7	0.421	11.5	LOS A	3.1	22.1	0.64	0.77	27.9
		410	2.2	0.421	8.5	LOS A	3.1	22.1	0.64	0.69	29.5
Approac		1015	1.6	0.421	7.7	LOSA	3.1	22.1	0.64	0.63	34.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 2021_S2_I-54 Susan Street/Beatrice Street-PM_AdjCounts

Beatrice Street_Susan Street: Roundabout: 2013

Roundabout

Mov ID	Turn	Demand	HV D	leg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	Sec		veh	m		per veh	km/h
South: S	Susan S	treet (S)									
1	L	24	0.0	0.184	8.1	LOS A	1.1	7.6	0.58	0.69	38.6
2	Т	141	0.0	0.184	7.3	LOS A	1.1	7.6	0.58	0.63	38.6
3	R	2	0.0	0.184	11.4	LOS A	1.1	7.6	0.58	0.81	36.1
Approac	ch	167	0.0	0.184	7.4	LOS A	1.1	7.6	0.58	0.64	38.6
East: Be	eatrice S	treet (E)						and the second			
4	L	16	0.0	0.233	7.6	LOS A	1.4	10.0	0.53	0.66	31.4
5	Т	206	0.5	0.233	6.8	LOS A	1.4	10.0	· 0.53	0.61	31.7
6	R	6	0.0	0.233	11.0	LOS A	1.4	10.0	0.53	0.80	29.1
Approac	h	228	0.4	0.233	7.0	LOS A	1.4	10.0	0.53	0.61	31.6
North: S	usan St	reet (N)									
7	L	1	0.0	0.191	6.4	LOS A	1.2	8.3	0.35	0.54	34.7
8	Т	69	0.0	0.191	5.5	LOS A	1.2	8.3	0.35	0.47	35.3
9	R	155	0.0	0.191	9.7	LOS A	1.2	8.3	0.35	0.68	32.1
Approac	h	225	0.0	0.191	8.4	LOS A	1.2	8.3	0.35	0.61	33.0
Nest: Be	eatrice S	Street (W)									
10	L	204	1.0	0.280	6.7	LOS A	1.9	13.6	0.43	0.57	31.2
11	т	56	1.8	0.280	5.8	LOS A	1.9	13.6	0.43	0.51	31.7
12	R	62	0.0	0.280	10.0	LOS A	1.9	13.6	0.43	0.70	29.1
Approac	h	322	0.9	0.280	7.2	LOS A	1.9	13.6	0.43	0.58	30.8
All Vehic		942	0.4	0.280	7.5	LOS A	1.9	13.6	0.46	0.61	33.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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SUSAN STREET / BEATRICE STREET MOVEMENT SUMMARY Site: 2021_S2_I-55 Susan Street/Helena Street_AM

Helena Street_Susan Street: Roundabout: 2013

Giveway / Yield (Two-Way)

Mov ID	Turn	Demand	HV E	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	vic	sec		veh	m		per veh	km/h
East: He	elena St	reet (E)							1947 - 1955 - 14 1947 - 1955 - 14	a per la	-
5	Т	151	0.7	0.135	1.0	LOS A	0.8	5.7	0.39	0.00	43.9
6	R	80	1.3	0.135	7.4	LOS A	0.8	5.7	0.39	0.72	41.2
Approac	h	231	0.9	0.135	3.2	NA	0.8	5.7	0.39	0.25	42.9
North: S	usan St	reet									
7	L	40	2.5	0.053	7.4	LOS A	0.1	1.0	0.29	0.60	28.7
9	R	31	0.0	0.066	11.7	LOS A	0.2	1.7	0.53	0.77	34.6
Approac	h	71	1.4	0.066	9.2	LOS A	0.2	1.7	0.39	0.67	32.4
West: H	elena S	treet (W)									
10	L	153	0.7	0.138	6.4	LOS A	0.0	0.0	0.00	0.71	30.8
11	Т	106	3.8	0.138	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approac	h	259	1.9	0.138	3.8	NA	0.0	0.0	0.00	0.42	37.5
All Vehic	les	561	1.4	0.138	4.3	NA	0.8	5.7	0.21	0.38	40.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 2021_S2_I-55 Susan Street/Helena Street_PM

Helena Street_Susan Street: Roundabout: 2013

Giveway / Yield (Two-Way)

Mov ID	Tum	Demand Flow veh/h	HV Deg. Satn		Average Delay	Level of Service	95% Back of Queue		Prop.	Effective	Average
							Vehicles	Distance	Queued	Stop Rate	Speed
			%	v/c	sec		veh	m		per veh	km/h
East: He	elena St	eet (E)		And Destroy			10000		17	a da sertar a	P. St. Const.
5	Т	231	0.9	0.165	0.8	LOS A	1.0	7.2	0.36	0.00	44.4
6	R	67	0.0	0.165	7.2	LOS A	1.0	7.2	0.36	0.73	41.4
Approach		298	0.7	0.165	2.3	NA	1.0	7.2	0.36	0.17	43.7
North: S	Susan St	reet									
7	L	43	0.0	0.056	7.2	LOS A	0.2	1.1	0.26	0.59	28.9
9	R	54	5.6	0.125	13.0	LOS A	0.5	3.4	0.57	0.83	33.6
Approach		97	3.1	0.125	10.4	LOS A	0.5	3.4	0.44	0.72	32.3
West: H	elena St	reet (W)									
10	L	95	1.1	0.108	6.4	LOS A	0.0	0.0	0.00	0.74	30.8
11	Т	110	0.0	0.108	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approach		205	0.5	0.108	3.0	NA	0.0	0.0	0.00	0.34	39.8
All Vehicles		600	1.0	0.165	3.8	NA	1.0	7.2	0.25	0.32	41.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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South Auburn Planning Proposal—Traffic, Transport and Accessibility Study

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